# Stave Falls Aquifer Hydrogeologic Review

City of Mission

December 2023

# **Quality Information**

Prepared by

Reviewed by

Aspen Anderson, Ph.D., E.I.T.

Hydrogeologist

Giulio Scarzella, M.Sc., P.Geo. (AB)

Senior Hydrogeologist

Reviewed / Approved by

Ryan D. Mills, M.Sc., P.Geo.

Senior Hydrogeologist

# Revision History

Revision	Revision Date	Details	Authorized	Name	Position
1	December 1, 2023	Draft	RM	Ryan Mills	Senior Hydrogeologist
2	December 12, 2023	Final	RM	Ryan Mills	Senior Hydrogeologist

#### **Distribution List**

# Hard Copies	PDF Required	Association / Company Name
0	Yes	City of Mission
0	Yes	AECOM

# **AECOM Canada Ltd.**

Permit to Practice No. 1001307

# **Table of Contents**

1	Intro	duction	1
	1.1	Initiation	1
	1.2	Background	
	1.3	Objectives	
_	1.4	Scope of Work	
2	Phys	sical Setting	
	2.1	Climate	2
	2.2	Topography and Drainage	
	2.3	Geology	
3		I Aquifer Characterization	
3		Mapped Aquifers	
	3.1 3.1.1	Mapped Bedrock Aquifers	
	3.1.2	Mapped Unconsolidated Aquifers	
	3.1.2	Other Aquifers	
	3.1.3	Hydrostratigraphic Units	
	3.2.1	Bedrock Aquifers	
	3.2.2	Unconsolidated Materials	
	3.2.2	Conceptual Hydrostratigraphic Cross Sections	
		Initial Geologic Modeling Framework	
	3.2.4	Hydrogeologic Properties	
	3.3 3.4	Hydrogeology	12
	3.4.1	Groundwater Flow	
	3.4.2	Existing Groundwater Use and Well Yield	
	3.4.3	Water Quality	
4		ping Level Water Balance	
	4.1	Methodology	
	4.2	Results	16
5	Vuln	erability of Groundwater to Contamination	17
	5.1	Methodology	17
	5.2	Results	19
6	Priva	ate Well Policy Review	
	6.1	Well Policy	21
	6.2	Form F-3 Private Well Certification	
_	6.3	Guidance for Detailed Reports for Private Wells – Domestic Use	
7		clusions	
8		ommendations	
9	Refe	rences	26
Figu	res (	In-Text)	
		e Falls Neighborhood Geology (Digitized from the GeoMap of Vancouver; NRC 1998)	
		Use According to the Zoning Code Within the Stave Falls Neighborhood	
		e Falls Neighborhood Bedrock Aquifers	
Figure [	D. Stave	e Falls Neighborhood Unconsolidated Aquifers	9
		to South Conceptual Cross Sections.	
		to West Conceptual Cross Sections.	
		ndwater Elevations in SFN	

## 1 Introduction

#### 1.1 Initiation

The City of Mission (City) issued a Request for Proposals (RFP 2023-006) for Consulting Services for Stave Falls Aquifer Hydrological Review. The review was to consider the aquifer or aquifers underlying the Stave Falls Neighborhood (SFN). AECOM submitted a proposal dated May 23, 2023 was accepted by the City and the contract was executed on June 28, 2023.

# 1.2 Background

The SFN is nestled on the City's western municipal boundary, approximately 9 km northwest of city centre (**Figure 1-1**). The SFN is flanked by the City of Maple Ridge municipal boundary to the west, the Fraser River to the south, and Stave River and impoundments to the east. The northern boundary of the SFN lies between Rolley Lake Provincial Park and Devil's Lake.

There are two hydroelectric dams on Stave River along the eastern boundary of the SFN. The northern hydroelectric dam, Stave Falls Dam, separates Stave Lake to the north and Hayward Lake to the south. Hayward Lake is bounded to the south by the Ruskin Dam, with the Stave River flowing south from the dam toward the Fraser River.

# 1.3 Objectives

As stated in the RFP, the objective of the review is to provide a "concise understanding with respect to the limitations of the aquifer(s), whether there is potential to add users into the aquifer(s) through subdivision or rezoning, and recommended changes to current private well policies, monitoring, and management."

# 1.4 Scope of Work

AECOM has conducted this review based on publicly available data and information provided by the City. The scope of work included the following:

- Review of local area geology, including available maps and plans of topography, surface geology, aerial
  photographs.
- Review of existing groundwater supply investigation reports and published data including water well logs/reports,
  Provincial observation well records, aquifer mapping reports and any other publicly available relevant data.
  Review of the current City of Mission private well policy available at: <a href="https://www.mission.ca/wp-content/uploads/Potable-Water-Supply-Form.pdf">https://www.mission.ca/wp-content/uploads/Potable-Water-Supply-Form.pdf</a>. Documentation of available water quality data and identification of potential groundwater contamination hazards in the study area.
- Mapping of septic system locations based on information provided by the City. Preparation of this technical report which includes the following:
  - Estimation of the capacity of the aquifer(s) underlying the SFN
  - Summary of the number of current licensed allowances
  - Estimation of the safe maximum extraction capacity of the aquifer(s) and associated number of users
  - Recommended changes to the City's current private well policies.
  - Recommended further assessment and monitoring programs to ensure best management of the aquifer(s)

Within the middle and eastern portions of the SFN, bedrock is overlain by overburden (Figure A). Overburden in the SFN mainly consists of fine-grained materials including till, silt and clay units, and gravel and sand. At ground surface, till generally covers northern half of the SFN while the south is generally composed of silt and clay. A gravel and sand unit underlies the till and silt and clay units. The gravel and sand outcrops as a band east of Iron Mountain along Wilson Street (NRC 1998).

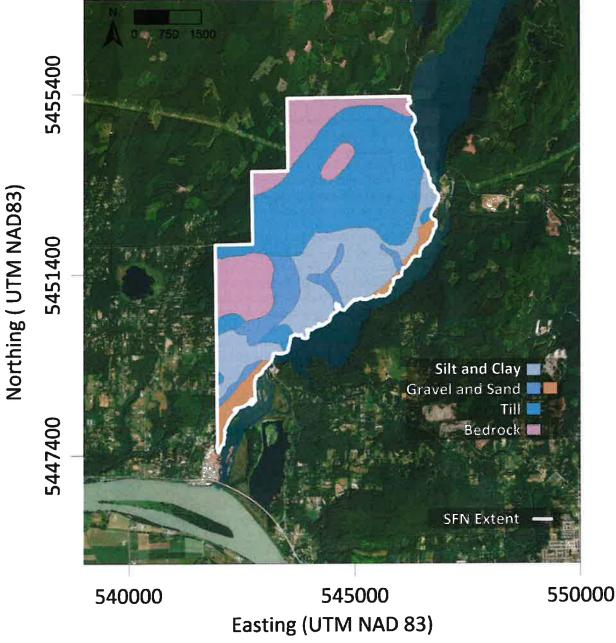


Figure A. Stave Falls Neighborhood Geology (Digitized from the GeoMap of Vancouver; NRC 1998)

#### 2.4 Land Use

According to the City of Mission's zoning codes (**Appendix B**), land use within the SFN, is 55.5% rural (**Figure B**; **Table C**). Institutional/Commercial Park, Open Areas, or Recreational Sites make up 35% of the land use. Residential land use that is primarily rural makes up 6.1% of the SFN, agricultural land makes up 2.7%, and commercial development makes up 1.6%. All of the agricultural and commercial development land is located south of Dewdney Trunk Road while the majority of the residential land is north of Dewdney Trunk Road. At the time of this report, the two commercial development parcels are largely undeveloped or are being utilized for residential development.

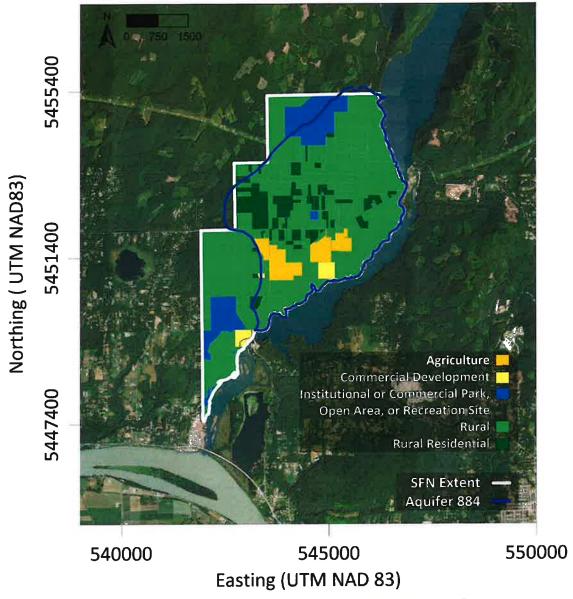


Figure B. Land Use According to the Zoning Code Within the Stave Falls Neighborhood

#### 3.1.1 Mapped Bedrock Aquifers

There are two mapped bedrock aquifers within the SFN that have identified: Aquifer 154 and Aquifer 19.

#### **Aquifer 154**

Aquifer 154 is a crystalline bedrock aquifer composed mainly of quartz diorite but can also contain granitic, volcanic, metamorphic, and sedimentary rocks. The aquifer is mapped as IIIB, which means it is lightly developed and has a moderate vulnerability (MWLAP 2002). The aquifer underlies most of the SFN except for portions of its northern and southern extents. It is generally confined except where it outcrops, which increases its vulnerability. These areas also serve as recharge zones that replenish the aquifer with direct infiltration and precipitation. The aquifer is also recharged by lateral groundwater flow from upland areas. Groundwater flow likely follows surface topography and is inferred to be to the east and southeast. No groundwater quantity or quality concerns are noted.

#### **Aquifer 19**

Aquifer 19 consists of fractured sedimentary bedrock in association with old sedimentary basins and is named the Grant Hill Aquifer. Hydrogeological information obtained from aquifer mapping reports indicates this unit is comprised of the Kitsilano Formation, which is a fractured sedimentary bedrock unit consisting mainly of sandstone and shale. The aquifer is classified as IIB which means it is moderately developed and has a moderate vulnerability (MWLAP 2002). The aquifer covers a small (approximately 1.4 km²) portion of the study area in southern portion of the SFN. A till layer generally covers the bedrock surface, and the aquifer is generally confined but appears to outcrop in areas west of SFN near Grant Hill and another bedrock high in the region. The main recharge area is inferred to be near Grant Hill, with radially outward groundwater flow ultimately discharging to the Fraser River. The eastern portion of the aquifer may discharge into the Stave River. Minor issues associated with water quantity and water quality have been reported but are judged to be anomalies in the data set.

Information on both aquifers is limited to groundwater use (discussed below) as no aquifer properties are known. Both aquifers are identified as low productivity and serve as minor water supplies for the SFN.

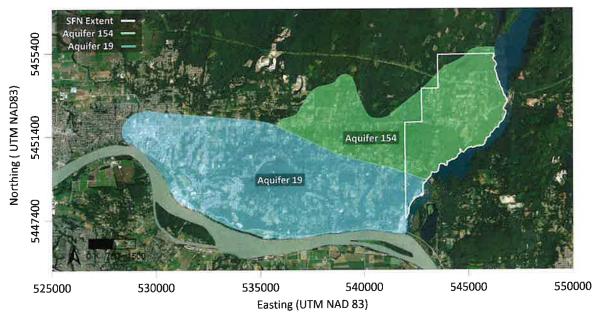


Figure C. Stave Falls Neighborhood Bedrock Aquifers

# 3.1.2 Mapped Unconsolidated Aquifers

There are three mapped unconsolidated aquifers within the SFN that have identified: Aquifer 884, Aquifer 26 and Aquifer 971.

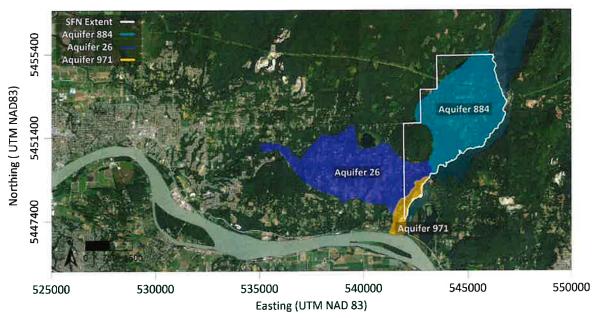


Figure D. Stave Falls Neighborhood Unconsolidated Aquifers

# 3.2 Hydrostratigraphic Units

Hydrostratigraphic units are defined as hydraulically continuous, mappable, and scale-independent entities. Based on available data, there are three hydrostratigraphic units within the SFN:

- Unconsolidated Materials: This unit is subdivided based on the interpreted permeability and saturated thickness into confining units (aquitards) and unconsolidated aquifers.
  - Unconsolidated Aquitards or Confining Units: These units are typically comprised of the low permeability till
    unit and the silt and clay unit.
  - Unconsolidated Aquifers: These units are typically comprised of the relatively permeable sand and gravel.
- Bedrock Aquifers: These units are primarily comprised of fractured crystalline bedrock and fractured sedimentary bedrock.

#### 3.2.1 Bedrock Aquifers

The top of the bedrock hydrostratigraphic unit (**Figure 3-2**) was delineated by spatially interpolating the "depth to top of bedrock" measurements (**Figure 3-3**) included in GWELLS and subtracting the bedrock depth from the elevation of the topographic surface (**Figure 2-2**).

#### 3.2.2 Unconsolidated Materials

The aquifer mapping reports and interpreted geology indicates that unconsolidated material in the SFN generally consists of a confining unit (comprised of till in addition to silt and clay) overlying aquifer sediment (comprised of sand and gravel). The Aquifer Mapping Report for Aquifer 884 and the accompanying Fact Sheet indicates that the aquifer is confined by fine-grained materials evident in all but four boreholes correlated to this aquifer. These resources suggest that the confining unit thickness ranges 0 to 72.5 m, with an average thickness of 21.9 m.

To develop unconsolidated material surfaces, we used GWELLS data to understand: 1) confining unit maximum depth at each well and 2) maximum thickness of unconsolidated material (i.e. depth to bedrock) at each well. We used values calculated from Equation 1 below as the basis for interpolation across the SFN and surface development:

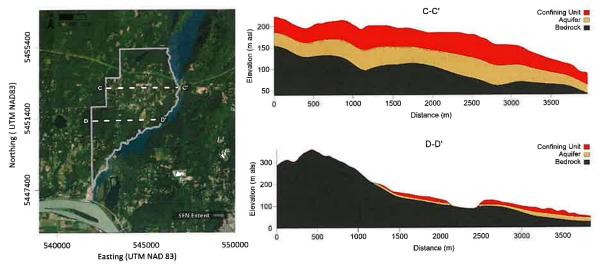


Figure F. East to West Conceptual Cross Sections.

### 3.2.4 Initial Geologic Modeling Framework

As a value-added service, an initial geological modeling framework was developed within Leapfrog™ to refine the SFN conceptual geologic model. Leapfrog™ is a geological modeling software that aids in the visualization of 3D geologic and hydrogeologic data. The software is the industry standard for analysing spatial data and creating 3D geologic models.

The initial geologic modeling framework included the compilation of GWELLS lithology data, topographic elevations, bedrock elevations, confining unit-aquifer contact surfaces, and satellite imagery (**Figure 3-7**). Lithology data contained in the GWELLS database is highly variable and does not follow a unified logging protocols, making it difficult to interpolate lithological or hydrostratigraphic units without extensive pre-processing. GWELLS data was simplified using a Python code developed for the SFN to recharacterize the drilling descriptions in GWELLS into common lithological terms and the associated hydrostratigraphic unit (**Appendix C**).

Results from this recharacterization of drilling data identified questions about the extent and continuity of the confining unit throughout the SFN. The geologic model may be advanced in the future to refine the geologic interpretation and resolve uncertainties surrounding the confining unit and aquifer outcrops.

# 3.3 Hydrogeologic Properties

Aquifer depth and thickness were discussed in earlier sections of this report. Other hydrogeologic property data including hydraulic conductivity, transmissivity, and storativity, were limited to one report for a new subdivision at 30782 Dewdney Trunk Road. Well data from GWELLS in the SFN were devoid of aquifer property data.

Several single-well pumping tests in well IDs 51761, 63652, 63687, 40652, and 63686 are assumed to have been completed in Aquifer 884 (**Table E**). The tests were conducted and analyzed by Active Earth Engineering Ltd. in 2021 (AEE 2021). While results were provided in the report, no analysis was provided to indicate the basis for testing and aquifer property determination. This information may have been included in a separate appendix or may not have been reported.

Table E. Summary of Reported Aquifer Properties - 30782 Dewdney Trunk Road, Mission

Aquifer Property	Units	Estimated Range
Transmissivity	m²/s	1.2 x 10 <sup>-4</sup> – 3.5 x 10 <sup>-3</sup>
Hydraulic Conductivity	m/s	2.3 x 10 <sup>-5</sup> – 6.9 x 10 <sup>-5</sup>
Storage Coefficient	unitless	4.2 x 10 <sup>-8</sup> – 5.9 x 10 <sup>-6</sup>

## 3.4.2 Existing Groundwater Use and Well Yield

Hydrogeologic information for each aquifer is derived from GWELLS and summarized in the Aquifer Mapping Reports and Fact Sheets. Data from these sources are summarized for mapped aquifers within the SFN below. Some of the values are approximated as described in the notes (**Table F**).

There is only one water use license in this area issued for domestic water supply use at 30259 Dewdney Trunk Road, Mission. The well was finished in Aquifer 884 with well diameter of 6", finished depth of 26 m, and reported well yield of 50 U.S. gallons per minute (GPM).

Table F. Groundwater User Data Summary.

Aquifer Property	Aquifer 884	Aquifer 26	Aquifer 971	Aquifer 154	Aquifer 19
Aquifer Type	Surficial	Surficial	Surficial	Bedrock	Bedrock
Confinement	Confined	Confined	Unconfined	Confined	Confined
Maximum Well Yield	3.8	18.3	1.9	≈ 0.08	≈ 0.03
Minimum Well Yield	0.03	0.06	0	≈ 0.6	≈ 0.6
Median Well Yield	0.9	0.6	0.6	0.2	0.2
Geometric Mean, Well Yield	0.8	0.6	0.3	0.2	0.2
Maximum Water Depth	97.5	95.4	24.4	88.1	182.9
Minimum Water Depth	0.3	0.3	1.5	Artesian	0.9
Median Water Depth	9.1	9.1	7.0	20.7	19.8
Geometric Mean, Water Depth	9.1	6.1	7.0	14.9	18.3
Maximum Well depth	131.7	106.4	≈ 24.5	194.8	286.5
Minimum Well depth	1.5	0.6	≈ 12.0	31.1	2.7
Median Well depth	21.3	23.2	19.2	97.5	93.3
Geometric Mean, Well Depth	20.1	14.0	NA	94.5	86.0

#### Notes:

Aquifers underlying the SFN are utilized for a range of purposes but are predominantly for domestic use (**Figure 3-1**). Reported well yields in the SFN (**Figure 3-8**) are summarized in **Table F**, which shows a wide range of reported values for each aquifer.

Available but limited groundwater user distribution data in SFN aquifers is described below:

#### Aquifer 884:

- 98 wells are installed in Aquifer 884, with finished depths between 3.8 to 129 m.
- 40 wells are reported as domestic, 1 as commercial / industrial and 5 wells belong to a water supply system.
   One of them is referenced as "Rolley Lake Water Supply System".

#### Aquifer 26:

Two (2) wells are installed within Aquifer 26, one of which has the finished depth of 106 m bgs and a well yield of 10 US GPM for domestic water use while the other well was finished with depth of 41 m bgs and a well yield of 3 US GPM for unidentified water use.

#### Aquifer 971:

 Two (2) wells are installed within Aquifer 971, one of which had a finished depth of 13.7 m bgs for unidentified water use but was abandoned. The other well was finished with unidentified well construction for domestic water supply.

<sup>1)</sup> Units: Aquifer area (km²), Well yield (L/s), Well and water depth (m bgs)

<sup>2)</sup> Some discrepancies were noted between data values provided in Fact Sheets and Reports. Data above is generally taken from Aquifer Mapping reports. Values with ≃ indicate estimated values from Fact Sheets.

$$P = Ro + ET + R$$

**Equation 3** 

In this equation, precipitation is the primary water input into the surface water system while runoff, evapotranspiration, and recharge are outputs leaving the surface water system. The groundwater balance considers recharge as the primary input into the groundwater system and groundwater discharge (Gd) leaving the system.

$$R = Gd$$

**Equation 4** 

Combining the surface water and groundwater balance equations, **Equation 5** shows the scoping level water balance considered in this analysis:

$$P = Ro + ET + Gd$$

Equation 5

#### **Precipitation**

Precipitation (P) data was obtained from the Canadian Climate Normals (**Table A**). Canadian Climate Normals up to 2010 were retrieved from Station 1107680 ("STAVE FALLS") as discussed in **Section 1.2**.

#### Runoff

Runoff (RO) is approximated monthly using a runoff coefficient method based on land cover to reflect the ratio of rainfall that results in surface runoff. Runoff coefficients were assigned to each zoning category (Table C) as a proxy for land cover within the SFN (Figure B). Overall, runoff coefficients assigned to the SFN are low, signifying the relatively high proportion of infiltration compared to runoff and reflecting the rural landscape within the SFN. Increased runoff coefficients representing lower infiltration and higher runoff were assigned to a commercial development zone due to the higher likelihood for pavement and other non-permeable material. A runoff coefficient of 0.19 for the SFN was developed by considering area weighted coefficients for each land use in Table G. This method does not consider topographic variability throughout the SFN, but this represents for future refinements to the runoff estimation and water balance.

Table G. Runoff Coefficients for Each Zone Type

Zone Type	Percent of Total SFN Area (%)	Runoff Coefficient Range	Assigned Runoff Coefficient
Rural Residential	6.1	0.3 - 0.5	0.3
Rural	55.5	0.1 - 0.25	0.17
Institutional or Commercial Park, Open Area, or Recreation Site	34.0	0.1 - 0.25	0.17
Commercial Development	1.6	0.5 - 0.9	0.7
Agriculture	2.7	0.2 - 0.5	0.35
Weighted Average	100	0.1 - 0.9	0.19

#### Evapotranspiration

Potential Evapotranspiration (ET) was estimated using a well-known analytical equation (following the Thornthwaite, 1948 methodology). The original Thornthwaite method calculated monthly PET based on average daily temperature, the number of days within the month, the average number of sunshine hours, and a heat index, which is dependant on the 12 monthly mean temperature. Day length data were not available directly from Environment Canada climate stations, so a latitude correction (of 49.2 degrees N) to the Thornthwaite method was applied instead.

#### **Groundwater Discharge**

Groundwater discharge (Q) from the surficial aquifers underlying the SFN to Stave Lake and Hayward Lake was approximated using the Darcy equation (**Equation 6**):

		Water Balance Components													
Month	Precipitation (m³)	Potential ET (m³)	Runoff (m³)	Groundwater Discharge (m³)	Inflow - Outflow (m³)										
July	1,653,997	2,345,605	0	88,553	-780,161										
August	1,647,946	2,164,899	0	88,553	-605,506										
September	2,071,531	1,621,807	85,448	88,553	275,723										
October	4,752,217	865,082	738,556	88,553	3,060,026										
November	7,479,296	389,914	1,346,983	88,553	5,653,846										
December	5,680,069	160,083	1,048,797	88,553	4,382,636										
Year	47,592,767	13,127,917	6,777,948	25,859,520	26,624,264										

The current scoping level water balance does not account for any change in water storage throughout the SFN and should be considered an order-of-magnitude estimate that requires confirmation with additional field investigation, testing, desktop analysis and monitoring.

Specific considerations for improving this scoping level water balance include:

- <u>Climate Data</u>: Climate normals used in this water balance were developed for 1981-2010 and have not recently been updated. More recent climate normals combined with local climate station measurements are needed to assess the current inflows and outflows within the SFN.
- Interactions with Surface Water Features: The degree of aquifer interaction with the surrounding water bodies, including Stave Lake, Hayward Lake, and the Fraser River, is uncertain. These water bodies may significantly interact with SFN aquifers and affect the overall water balance calculation. This interaction needs to be quantified through drilling, monitoring, and testing to support further analysis.
- Extent and Continuity of the Confining Unit: Recharge entering the groundwater system through infiltration from ground surface is expected to be reduced or delayed in areas where the confining unit is present and thick.
- Spatial Extent of Recharge Areas: Identification, coverage extent and distribution of recharge areas within the SFN would benefit this analysis by leading to more accurate recharge estimates.
- <u>Groundwater Withdrawal:</u> Groundwater withdrawal data in the SFN is incomplete and/or limited. Detailed accounting of groundwater abstraction within the SFN to refine water balance estimates.
- <u>Climate Factored Analysis:</u> To be best prepared for future water management decisions, a climate-factored water balance is required. Climate factored precipitation and temperature data can be obtained from the Pacific Climate Impacts Consortium and NASA and should be applied in future water balance updates to ensure conclusions and recommendations are climate resilient.

# 5 Vulnerability of Groundwater to Contamination

# 5.1 Methodology

Vulnerability is defined in this report as a combination of the physical susceptibility of an aquifer(s) to groundwater contamination in the presence of a hazard or hazard threat, which is any stressor (natural or anthropogenic) that may act to adversely impact groundwater resources.

Similar definitions of vulnerability within integrated risk frameworks have been used in groundwater applications (Simpson et al. 2014 Holding and Allen 2016; Klassen and Allen 2017). Several of these studies were completed locally within southern British Columbia. We chose this definition of vulnerability over the DRASTIC method, which is commonly used, as it can account for many specific and known hazards within the SFN. The method employed in this study may be refined in the future upon further data availability to include some DRASTIC method components (such as topography, soil media, and vadose zone impact) within the calculation of aquifer susceptibility, where susceptibility in this report is analogous to intrinsic vulnerability defined within the DRASTIC method.

The resulting hazard map (**Figure 5-2**) is limited by available data sets and in this case any hazards present in SFN not identified or accounted for in **Table J** remain as data gaps. Other prominent hazards may include dry cleaners, gas stations, and unidentified industrial lands. In some cases, specific property activities may represent certain hazards (e.g. a homeowner has a large "shop" where mechanical work is completed). Due to data and scope limitations, our hazard analysis does not consider groundwater flow direction, the potential for downgradient impacts from upgradient sources and impacts from surface water - groundwater interactions.

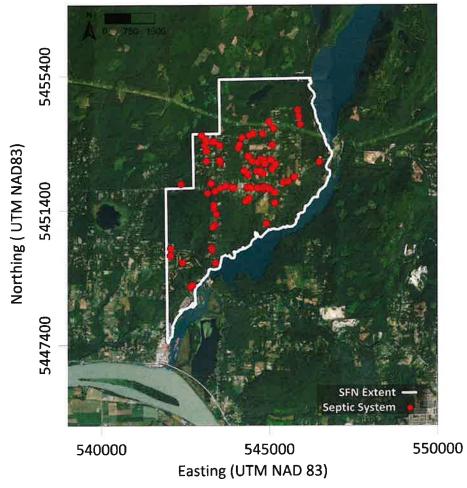


Figure H: Septic systems within the SFN.

#### 5.2 Results

Vulnerability to groundwater contamination (**Figure I**) is highest within the southern part of the SFN where susceptibility is high due to interpreted aquifer outcropping collocated with agricultural land or septic systems. The highest vulnerability within the SFN was interpreted along Dewdney Trunk Road where residents and agricultural lands are dispersed, along the northern part of Wilson Street where Aquifer 884 is expected to outcrop at ground surface, and along Wilson Street south of Ruskin Dam where Aquifer 971 is exposed at surface.

Comparing vulnerability to existing groundwater development indicates that there is already groundwater development within interpreted vulnerable areas along Dewdney Truck Road (**Figure 5-3**). High vulnerability areas may be used to identify future groundwater monitoring locations and/or inform policy decisions regarding zoning codes and development in highly vulnerable areas.

• More Comprehensive Water Level And Aquifer Property Data Sets: To account for potential downgradient impacts from upgradient sources, more detailed and current groundwater elevation information is needed in addition to larger aquifer property data sets. With strong data sets, the accuracy of groundwater discharge, infiltration rates and gradients can be improved. Further, a three-dimensional numerical groundwater flow model may also be used to identify source-receptor pathways, establish a robust water balance for the aquifer and identify areas that should be protected to avoid contamination of a wellfield or the aquifer.

# 6 Private Well Policy Review

We have reviewed the *Potable Water Supply – Rural Subdivisions & Building Permit Application* document issued by City of Mission Development Services and the associated documents. While the document is generally clear, it is also highly focused. As the SFN and the City of Mission continues development, refinements to the Well Policy may be required.

## 6.1 Well Policy

Our comments are summarized below on a section-by-section basis together with key recommendations for modifications to the document. It is recommended that the City's in-house legal council also review the document before it is updated and issued.

#### General:

The objective of the Well Policy is clearly stated. There is a mandatory requirement for the owner of a subdivision to prove a potable water supply by way of a private well for each lot prior to approval of a subdivision. The third paragraph could be shortened for conciseness. The end of the paragraph contains verbiage from Bylaw 56509-2017 Section 3.15.2, which is redundant as the previous sentence indicates that conformance with all requirements outlined in Section 3.15 is required.

Policy Recommendation #1: Change this paragraph to the following: "All new lots must be serviced by drilled or dug wells and must be tested and certified in accordance with the City of Mission Development and Subdivision Control Bylaw 5650-2017 (as amended), Section 3.0–Water Distribution, 3.15 Private Water Systems. Groundwater use is governed by the provincial government and an additional reference to the provincial acts and regulations governing groundwater use and licensing should be added as follows: "The use of groundwater is governed by the Water Sustainability Act, Water Sustainability Regulation, and the Groundwater Protection Regulation, which establish the requirements for groundwater investigations, analysis and licensing in the Province of British Columbia. It is recommended that all developers and groundwater users consult these documents for additional information in advance of investigating a groundwater and/or surface water supply".

#### **Building Permits:**

This section clearly states a completed private well certification form is required at application stage for any property without municipal water, and appropriately describes the relationship to other municipal approval processes.

**Policy Recommendation #2:** Consider providing a flowchart or table that lists all required approvals and the sequencing of document submission, municipal reviews and approvals.

#### Detailed Report on Water Quantity, Water Quality, and Hydrogeological Impact Assessment and Form F-3:

Clearly states requirements that a report and F-3 Form are required and who must prepare the report. However, the Association of Engineers and Geoscientists of British Columbia (APEGBC) was renamed as Engineers and Geoscientists of British Columbia (EGBC) several years ago. Furthermore, the *Professional Governance Act* was implemented and requires firms to have a Professional Practice Management Plan (PPMP) in place as of September 30, 2021. The document is to assign Responsible Registrants that are able to apply the firm's Permit to Practice to all technical documents.

**Policy Recommendation #3:** Update the reference to the professional association to be Engineers and Geoscientists of British Columbia (EGBC), and require the firms meet the requirements of the *Professional Governance Act* and all applicable EGBC Bylaws.

requested to comment on the viability of a single well. It is also important to recognize that one or several wells installed for a subdivision may not be the sole cause of unsustainable withdrawals from an aquifer or provide evidence of adverse impacts on their own. It may be the large number of wells in numerous subdivisions spread across the aquifer that may cause issues such as over pumping or water quality degradation, and these impacts are best evaluated with area-wide assessments conducted by government agencies. Anthropogenic activities and hazards may also harm aquifers following completion of documentation...

Many technical guidance documents have been developed by the provincial government to guide evaluation of the sustainability of groundwater supplies, including "Guidance for Technical Assessments in Support of an Application for Groundwater Use in British Columbia", with specific reference to Section 2.1: Assessing Adequacy of Supply, Section 2.2: Assessing Likelihood of Hydraulic Connection to Streams and Other Aquifers, Section 2.3: Assessing Potential Impacts on Nearby Groundwater Users, Section 3.5: Methodology for Assessing the Adequacy of the Supply, Section 3.6: Results Used for Assessing the Adequacy of the Supply, and Section 3.7.3: Assessment of Potential Impacts. While domestic groundwater use evaluations are exempt from many of the requirements,

**Policy Recommendation #5:** It is recommended that the City of Mission add the following phrase: "The withdrawal of the above daily quantities of water has been conducted in a manner that meets the requirements of the Technical Assessment Guidelines (Todd et al., 2020), and is judged to be able to provide those quantities of water at all times of the year without impacts to existing groundwater and/or surface water users. Furthermore, the impact of climate change on the long-term groundwater extraction has been evaluated in accordance with the requirements of EGBC and all applicable provincial acts and regulations, and the above quantity of water is judged to be sustainable in the context of known existing groundwater users".

The remaining certifying statements regarding water quality are judged to be reasonable and clear.

# 6.3 Guidance for Detailed Reports for Private Wells - Domestic Use

As stated, "This guidance is intended for professional engineers and geoscientists in the preparation of detailed reports for submission to the City of Mission so as to meet the minimum information requirements of a "detailed report" as referenced in Section 3.15 of Schedule C of the City of Mission Development and Subdivision Control Bylaw 5650-2017 (as amended). The detailed report must be for one well only, and each report must be signed and sealed by a registered Professional Engineer or Geoscientist with experience in hydrogeology. To meet the definition of "experience in hydrogeology", the professional must be registered with Engineers and Geoscientists BC as having a primary or secondary field of expertise in hydrogeology or as a hydrogeologist".

This document contains many statements that duplicate and may contradict some of the statements made in provincial guidance documents. Professional engineers and geoscientists licensed with EGBC having expertise in hydrogeology and groundwater supply evaluations should be very familiar with provincial acts, regulations and the technical; requirements outlined in provincial guidance documents and policy. In aggregate, these documents establish industry standard in British Columbia. There are opportunities to simplify the guidance document by referencing provincial guidance documents, and focus on supplementary requirements of the City of Mission, and the noted exemptions.

**Policy Recommendation #6:** It is recommended that the City of Mission reference the requirements of the Technical Assessment Guidelines (Todd et al., 2020) for a list of technical assessment and reporting requirements. To recognize the full value of the analysis and reporting, the City of Mission should:

- Require that professionals provide a copy of pumping test analysis reports and an estimated hydraulic conductivity value for the aquifer.
- Require that professionals specify how, where and when the water quality samples were collected.
- Require that professionals provide justification for the methodologies employed in the analysis.

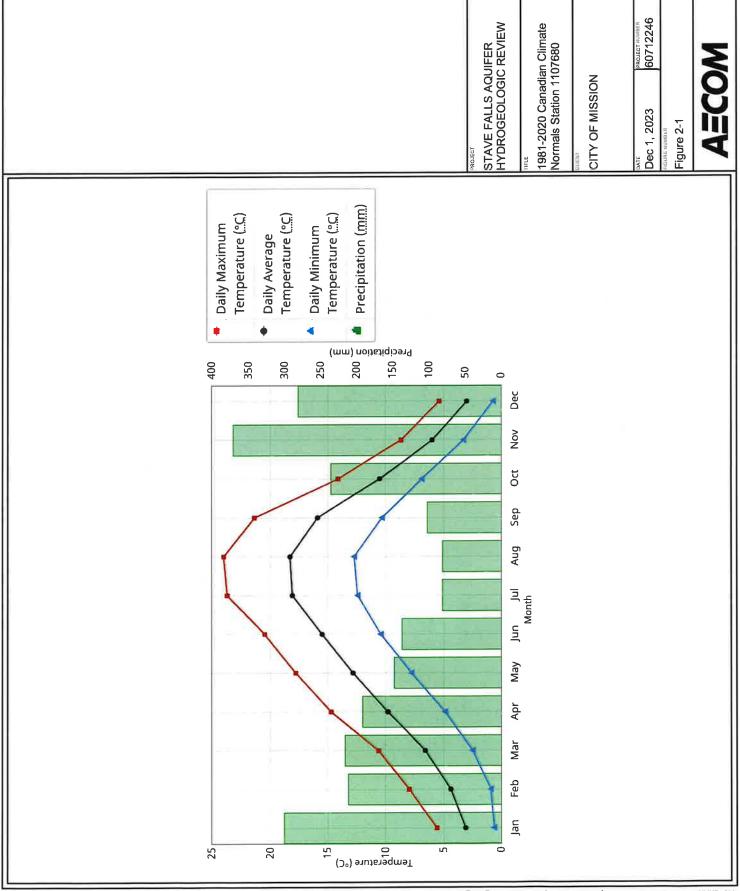
# 8 Recommendations

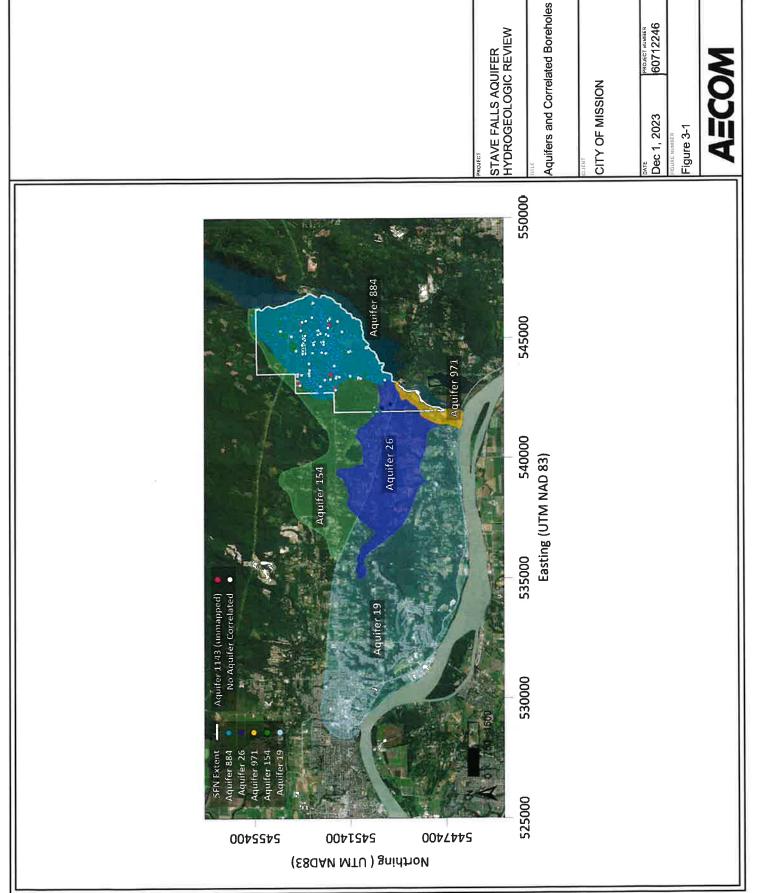
Detailed context for recommendations is interspersed through the document in relevant sections. The following recommendations are made to improve the overall hydrogeologic understanding in the Stave Falls Neighborhood to inform future policy decisions and ensure sustainable use of groundwater resources for the community:

- 1. Establish a Digital Database to house hydrogeologic data including well locations, water use/groundwater pumping data, groundwater level measurements, well evaluation reports, septic system locations, borehole logs/lithologies, groundwater chemistry, etc. Many hydrogeological analyses utilize geospatial and subsurface data that must be in digital format to be useful. Prospective developers and groundwater users should be required to provide digital data for upload of any new information into this database to streamline digitization and record keeping. Data can be expensive to digitize, so preservation of digital data when available is recommended.
- 2. Characterize Aquifer Properties and the Hydrogeologic Connection to Hayward Lake and Stave Lake to determine the sustainable yield of the unconsolidated aquifers underlying the SFN. Pumping tests are required to confirm aquifer properties (such as hydraulic conductivity) at a scale that is appropriate for a regional assessment. Completing pumping tests in targeted locations near Hayward Lake and Stave Lake would allow for quantification of groundwater/surface water interactions along the eastern boundary of the SFN, which is critical for future water balance evaluations and vulnerability assessments. This information is critical for ultimately determining how much groundwater resources are available for consumption.
- Implement a Groundwater Monitoring Program to monitor the current state of the aquifer systems and how they behave throughout the year in response to meteoric inputs and outputs, groundwater use and fluctuations in the elevation of Stave Lake and Hayward Lake. The monitoring system should include a series of monitoring wells in upland and lowland environments that are initially focused on Aquifer 884 and be monitored for water levels and water quality. Considering some residents within the SFN have experienced dry wells during some summer months, it is critical to create a monitoring program for regular data collection to diagnose these types of problems and monitor for any future issues that arise. Furthermore, the current understanding of groundwater quality is focused on point of use (tap water) data that may be influenced by household plumbing and water treatment systems. Monitoring programs produce the most reliable information when monitoring is conducted at the same locations by the same staff over a prolonged period. This is best completed in municipally owned wells.
- 4. Improve the Hydrogeological Conceptual Model to understand where the aquifer outcrops, characterize the extent and continuity of the confining unit, and determine the connection between the aquifers and Hayward Lake and Stave Lake. Data in the GWELLS database has been utilized to develop a preliminary geological model, but it is critical to ground truth the geological mapping through field investigation. Additional drilling is required to fill data gaps in targeted locations. Having a detailed geologic model is critical for all future hydrogeologic investigations and will support refinements of the initial water balance and vulnerability analysis.
- 5. **Establish a Local Meteorological** Station to monitor precipitation, temperature, relative humidity, net radiation, wind speed and wind direction within the Stave Falls Neighborhood. This is important information for establishment of the inputs (groundwater recharge) and outputs (evapotranspiration) from the water balance and is known to be highly variable in mountainous environments.
- 6. Consider the Impacts of Climate Change in future water balance evaluations to ensure the long-term sustainable aquifer yield is climate resilient. The Pacific Climate Impacts Consortium (PCIC) Climate Explorer can be used to develop future climate scenarios for the Stave Falls Neighborhood. Data from the proposed meteorological stating within the Stave Falls Neighborhood should also be used to validate model outputs. The Lower Mainland is forecast to experience longer and drier summers in conjunction with more intense fall precipitation events. Short duration extreme weather events like Atmospheric Rivers have already resulted in major flooding within the Lower Mainland and drier summers are resulting in water shortages and more intense forest fire seasons. It is critical to validate these predictions with climate analysis and prepare for future changes in water resources.
- Update the Private Well Policy to minimize duplicity and contradictions with established technical guidance documents and focus on information that is important to the City of Mission.

# **Figures**

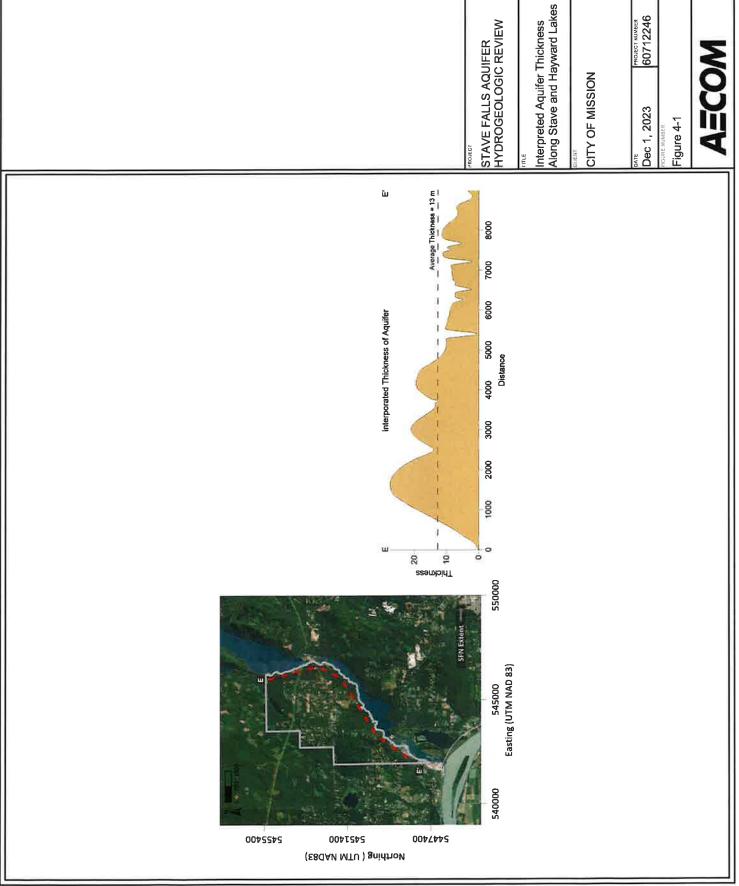
AECOM

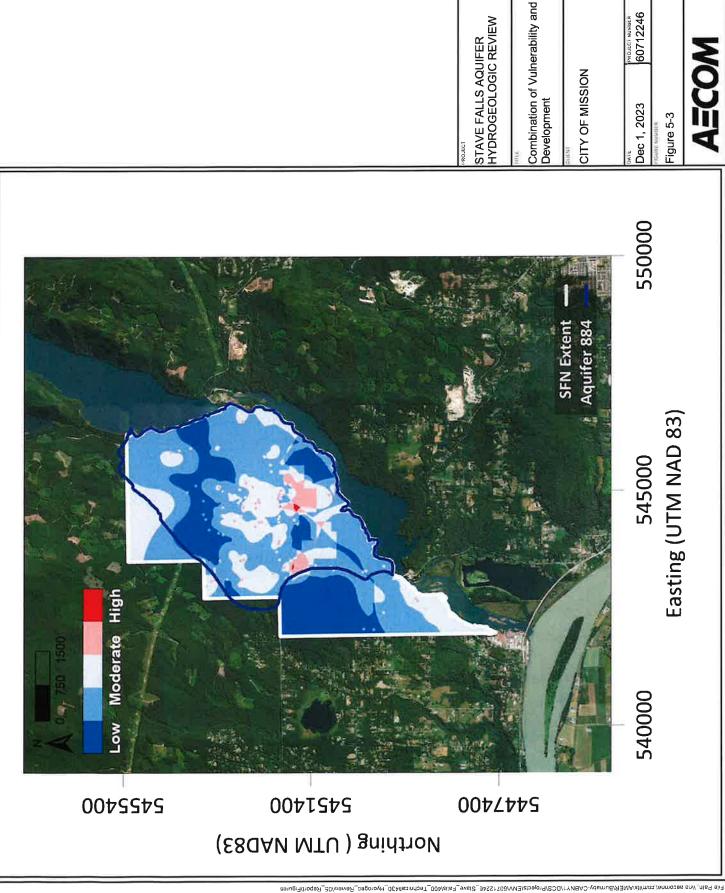




Initial Leapfrog Geologic Modeling Framework STAVE FALLS AQUIFER HYDROGEOLOGIC REVIEW CITY OF MISSION Dec 1, 2023 Figure 3-7 - East (X) +542000 +542500 +543000 +543500 +544000 +544500 +545000 +545500 +546000 +546500 Elev (

ряолест иливея 60712246





Stave Falls Aquifer Hydrogeologic Review

# Appendix B

2023 Zoning Codes

#### 2023 Zoning Codes

ID	Zone Class	DoM Zoning Description	AECOM Reclassification	Shape Area	Percent of Total Area
111	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	10453	0.03
112	RR7	Rural Residential 7 Zone	Rural Residential	3962	0.01
113	RR7	Rural Residential 7 Zone	Rural Residential	8884	0.02
114	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residentia	9575	0.02
115	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	9364	0.02
116	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	61091	0.16
117	RR7s	Rural Residential 7 Secondary Dwelling	Rural Residential	40150	0.10
118	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	20175	0.05
119	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	11268	0.03
120	RR7s	Rural Residential 7 Secondary Dwelling Zone	Rural Residential	20783	0.05
1844	10,112	TARREST CONTRACTOR OF THE PARTY			100.00

2023 Borehole Data Simplified

Well	Well Tag No	Date	рН	Turbidity	Total Coliforms	Escherichia coli	Aluminium	Arsenic	Iron	Lead	Manganese
-10	110				mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L
54752		2020-04-17			1				4.2		0,31
54752		2020-04-28			6.3				12		0.46
54753		2020-02-27									0.15
67421		2023-03-02							0.10	_	0.02
54720		2020-12-03		2.95					0.42		0.263
54721		2020-12-09		1,04							0.062
	78326				<1					_	
	78326	2022-04-06			6.3					_	
	124594	2021-12-09 2022-04-06			<1		-				
_	124594 124593	2021-12-09			12.1						
_	124593	2022-04-06			<1						
_	93361	2021-03-24									0,1
_	122289	2021-04-08							0.32		0.046
	122287	2021-03-03									0.035
	122288	2021-02-03			69,7						0.06
	122288	2021-02-19			<1						0.11
64074		2019-11-05			4.2						
64074		2020-12-09			<1						
67589		2022-02-25		0.8							0.065
61552		2020-11-04			7.5						0.032
67588		2022-02-25		0.57							0.044
40652		2020-11-02			>200.5	3.1					
40652		2020-12-08			<1	<1				0.015	0.033
64092		2022-03-03		2.25	11.1				0.73	0.013	0.033
51761		2020-10-28		5.44	3.1				0.73		0.071
67407	-	2022-05-19		0,6 2.86	144.5			_	0.81	_	0.093
63652		2020-10-27		2.00	17.8				0.01		7.77
63652 67406		2020-12-09 2022-05-13		0.34	17,0						0.092
63687	_	2020-10-29		0.04							0.022
40691		2022-05-05		0.63	2				0.94		0.2
63686		2020-10-30			32.4						
40689		2020-10-00		1.48							0.27
61568		2020-12-08		36.4	64.4		1.4		1.4		0.14
40690		2022-04-26		0.8							0.064
63699		2022-05-30		0.66					1.4		0.21
40632		2021-12-08		0.47							0.069
40634		2021-12-01		0.36						_	0.046
40633		2021-12-02						0,036	2		0.14
63725		2021-11-30		0,37				0.01		-	0.057
63677		2021-07-12		0.67	1			0.012		-	0.065
61540		2021-07-13		1.35						_	0.064
41582	_	2021-06-24		1,35					0.84	_	0.064
61595		2021-07-14 2021-06-25		4.68 1.55	547.5				4,44		0.063
65722 63677		2021-06-23		4.68	62.4						0.064
63721	$\vdash$	2021-06-23		0.22	3.1			0.012			0.025
61596		2021-08-13		0.39				0.011			0,035
63706		2021-08-10		0.48	1						0.063
63705		2021-06-23		0.28	- 1						0.063
64224		2022-03-01		1.05							0.063
64243		2022-03-01		0.79							0.057
41556		2015-12-15	5,98		1				3.29		
										-	
Notes:					1					_	-
Guidelin	es for Canada	an Drinking Water Q an Drinking Water Q	uaility - Maximu	m Allowable Conc is Objective (AO)	entration (MAC) E	xceeded					

Ø,
5
ŧ
g
ō
_
ç
ם
ŝ
8
ä
Ň
Ħ
.≝"
_
ន
0
N

Other						I										I											I								ľ																I	I					
DV Permit	DV19-006			DV15-011											2000	DA18-011													DV19-001	<null></null>															V17-031/DV17-0:				DV20-019								DV20-020
DP Number	DP19-051	DP21-103	DP18-058		DP20-071/DP20-072/DP20-073		DP18.081	DP23-022	DP16-010	DP18-033	DP23-061	DP21-155/DP21-156/DP21-157	DP19-099		DP19-032	DB2+ +37/DB2+ +38/DB2+ +38	ODDO DIT		DP22-023	DP23-005	DP23-068		DP17-034	DP22-024/DP22-025/DP22-026	DP19-059	tito at Colone at an	DE10-0/0/DF10-0//	DP22-074/DP22-075		DP21-078/DP21-079		DP20-053/DP20-054		DE02 027/DE23 028/DE22 020	DP18-128/DP18-129/DP18-130	DP21-111/DP21-112/DP21-113	DP18-078/DP18-079		***************************************	080 8100	DP21-069	DP22-012		DP20-086/DP20-087		DP22-011	CF0.5240101020.031100523.032	DP23-045/DP23-046/DP23-047	DP20-100	DP20-063/DP20-064/DP20-065	DP21-009	0018.018	DP22-010	DP17-040	DP18-005/DP18-006	DP19-058	DP20-107
OCP Amend Number																																																							OCP18-001		
Rezoning Number				R15-015	R20-020	K18-016	2000		R15-028	R18-027		R21-048		R18-019	2000	C10-01X							R17-035	R22-012		2000	R18-04/	R22-028			R16-035	R20-012	R15-003	003 000	R18-062	R21-036	R18-048	R15-008	R15-002				R22-051	R20-025		B18.008	R22.014	R23-012		R20-017				R17-042	R18-006		
Subdivison					S20-010	211.018			\$15-018	S18-015		S21-02B										-	\$17-019	S22-012		000	210-075	\$22-025					\$15-003	622.006	518-031	\$21-020	\$18.024		215-002							S18.007	522.013	\$23-008		820-008	1000	cun-/Ls	822-004	\$17-021	ONE CONTROL		
Comments	Variance for Retaining Wall	Fire Interface DP - Duplex	Environmental DP	Garden Cottage	5 lot subdivision	Out Notice of Subdiction	Gaorine Adiretment	Fire Interface DP	3 Lot Subdivision	4 Lot Subdivision	Fire Interface DP	2 lot subdivision	Fire Interface DP	Garden Cottage	Fire Interface DP	OBSCI LIGHT	Fire Interface DP - Accessory Building	Discharge of Covenant - Register New	Fire Interface OP	Natural Environmental DP	Fire Interface DP	Release of Septic Cov. and provide new	3 Lot Subdivision	4 lot subdivision	Fire Interface DP	Discharge of Covenant	Secondary Suite	2 lot subdivision	Variance for Setbacks - accessory building	Environmental & Fire Interface DP's	Garden Cottage	Garden Cottage - RAR DP - Fire Interface DP	2 Lot Subdivision	A lot subdivision	26 lot Subdivision	2 lot subdivision - RR7s	2 Lot Subdivision	Secondary Suite	2 Lot Subdivision	The interface OF 101 Coach nouse	Fire Interface DP	Environmental DP	Rezone for Secondary Dwelling	Coach House	Accessory Building	The interface DP	10 lot subdivision	4 lot subdivision	Variance to build SFD	11 lot subdivision & rezoning to RR7s	Fire Interface DP - Accessory Building	Lot Line Adjustment	2 lot subdivision	5 Lot Subdivision	Gas Station/Convenience Store	Fire Interface DP	Variance - Accessory Building Height
Address 2										11511 Wilson St																																				12085 Dilorim St	13085 Pilorim St				The second second	29585 Hudson Ave					
Address 1	#13 - 11540 Glacier Dr	#14-11540 Glacier Dr	10531 Ruskin Cres	10549 Reedal St	10911 Wison St	11020 255th St	11067 Wilson St	11150 288th St	11445 Wilson St	11533 Wilson St	11666 Allan St	11707 Wilson St	11764 Wilson St	11809 Statim St	11809 Statim St	11820 Stillim St	11930 Yeo St	12060 Coughilon Crt	12071 Rolley Lake St	12071 Rolley Lake St	12100 Coughlin Ct	12140 Coughlin Ct	12162 Rolley Lake St	12187 Rolley Lake St	12243 Rolley Lake St	12281 Beil St	12334 Dell 31	12358 Carr St	12370 Powell St	12370 Powell St	12411 Carr St	12454 Rolley Lake St	12550 Powell St	100 Massell Idil	12631 Bell St	12631 Carr St	12631 Powell St	12638 Cathy Cres	12654 Powell St	18 Haword #5021	12658 Powell St	12658 Powell St	12658 Powell St	12712 Cathy Cres	12913 Pilgrim St	12933 Bell St	13033 Pilonim St	13157 Pilgrim St	151	29317 Dewdney Trunk Rd	29546 Taise PI	29573 Hudson Ave	29599 Kennedy Terr		29622 Dewdney Trunk Rd	29622 Dewdney Trunk Rd	29622 Dewdney Trunk Rd
Applicant	Thor Shay	Eleven Eleven Hornes Ltd	Annika & Stewart Swingle	Flowerdew	OTG Developments	Kelly Brack	Jeff Timper	Wayne Lindberg	Orca Pacific Developments	OTG Developments	Daniel Ewart	Marty Nault	Lacey Developments	Hilda Goddard	Kriemhild Goddard	Janel Cox	Scott Wideen	William Coughlin	Wei Zhang	Wei Zhang	Formosa Homes Joint Venture	Ben Sidhu	Len Murdoch	Lafleur Developments	Kelly Molloy	BC Quality Surveyors	Lacey Davelopments Ltd	Mitchel Loor	Chad & Amy Hensbee	Chad Hensbee	Noian Woods	Sander Hunfeld	Holzapiel	Pacific reak notifies inc	FlorWest Construction	Gary Lowther	Bodena Ollen	Chad Swash	Bondana Oilen	Bodana Ollen	Marie Kran	Marie Krzus	Marie Krzus	Michael Widdows	Gamache	RayRidge Developments	OTG Developments 1 td	Harjot Singh Toor	Brent Lindberg	1209381 BC Ltd	Cavaller Homes Ltd	Deanna Garda	Clar Habussler Edward Grice	Heliofs & Gillespie	Eros Homes Lid	Veer Sidhu	Eros Homes Ltd
Year	2019	2021	2018	2015	2020	2018	2018	2023	2015	2018	2023	2021	2019	2018	2019	2000	2020	2023	2022	2023	2023	2023	2017	2022	2019	2022	2018	2022	2019	L	Н	4	2015	2000	2018	2021	2018	2015	2015	2018	2021	2022	2022	2020	2017	2022	2002	2023	2020	2020	2021	4	2022	+	1	2019	Щ
TM EastinTM Northin	5451019		$\vdash$	-	+	5450059	+	٠	4	-	-	Н	5451564	-	5451685		+	+-	-	5452089	-	-	-	-	-	-	5452638	-	-			5453026		_	-			-	-	5453540	+	+-	-	$\rightarrow$	4	5453855	+	+-	Н	1	+	+	5450914		5451980	1 1	5451980
	544912	544912	542647	542702	543394	542047	54327E	542059	543384	543415	545157	543336	543341	544290	544290	544587	545153	545729	544341	544341	545729	545729	544344	544336	544245	545082	943149	543538	543118	543118	543517	544366	543119	544072	545082	543519	543068	544109	543115	543080	542975	542975	542975	544180	545916	545117	545860	545834	542406	542365	543252	543292	543315	543351	T	543373	
Project Number	P2019-030	P2021-089	P2018-057	P2015-033	P2020-065	P2018-022	P2018-077	P2023-026	P2015-061	P2018-039	P2023-060	P2021-128	P2019-083	P2018-027	P2019-023	P2010-024	D2020-103	P2023-064	P2022-027	P2023-007	P2023-068	P2023-074	P2017-052	P2022-029	P2019-043	P2022-124	P2018-0/4	P2022-065	P2019-010	P2021-069	P2016-061	P2020-045	P2015-006	000000000	P2018-114	P2021-097	P2018-075	P2015-019	P2015-003	P.2019-001	P2021-063	P2022-011	P2022-116	P2020-082	P2017.061	P2022-008	D2022.031	P2023-047	P2020-096	P2020-058	P2021-007	P2017-020	P2022-012	P2017-066	P2018-007	P2019-042	P2020-101

AppE\_2023\_Digitized\_Septic\_Locations\_60712246.xls

aecom.com

